Airborne Digital Frame Camera Imaging for Elevation Determination

Doug King, Pat Walsh, and Frank Ciuffreda

Abstract

A high resolution digital CCD frame camera was flown over a test site in Toronto, Ontario to acquire overlapping images for elevation determination. A selected pair of 1320 by 1035 images covering a total of 0.91- by 1.04-km area with 0.69-m square pixels were analyzed. Twenty-four points within the images were field surveyed and a bundle adjustment algorithm was applied to perform relative and absolute orientations of the images using only three control points. The weighted average RMS errors of the residuals were 1.138m (x), 1.390m (y), and 0.927m (z) within a total elevation variation of 40m. This level of accuracy was obtained despite several limitations such as lack of camera calibration, difficulties in image point selection, and image motion. These results have encouraged further research to improve the technique and apply it in the development of large-scale elevation models.

Introduction

An emerging solid-state imaging technology which has not vet been extensively tested in airborne imaging is the digital frame camera (DFC). It is a relatively low cost technology which has several advantages over current video sensors, line scanners and photography. In relation to standard videography, DFCs are not confined by National Television Standards Committee (NTSC) or other television scanning specifications. Consequently, sensors have been developed which contain many more photosites per unit sensor dimension and their resolution has the potential to reach that of 35-mm or 70-mm photography (current sensors typically have more than 1024 by 1024 photosites while near-future sensors will contain up to 4000 by 4000 photosites). In relation to line scanning, DFCs have the capability for acquisition of instantaneous two-dimensional raster exposures of the Earth's surface which do not suffer from the line-to-line geometric variability which is typical of airborne line scanner imagery. Photogrammetric analysis and geometric transformations are therefore much simpler, being for the most part analogous to procedures used with photography. In relation to standard aerial photography, DFCs have better geometric stability (no film warping), better radiometric image quality (linear response, greater dynamic range, greater quantum efficiency, and wider spectral sensitivity range), and capability for real time in-flight viewing of acquired images. The digital image format is also more suited to image processing for im-

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provement of radiometric characteristics (e.g., noise reduction, reduction of brightness variations with view angle), and to quantitative image analysis. To-date, two such cameras have been tested in airborne data acquisition by the principal author: (1) the Kodak DCS camera, a 1280- by 1024-CCD frame sensor housed in a Nikon F-3 body, was compared to simultaneously acquired 70-mm photography and standard video (reported in Mausel et al. (1992)); and (2) the Kodak MEGAPLUS 1.4 camera was flown to acquire data for this research. The cost of DFCs varies between \$10,000 and \$25,000 depending on the sensor quality, class, and design. As costs continue to decrease, this sensor format will compete and replace standard airborne videography and small format photography. Great potential exists for their use as multi-purpose sensors in photogrammetric and multispectral applications. King (1992) is currently developing a multispectral DFC system for natural resource and environmental evaluation. elevation modeling, and close-range scanning.

Image readout from a CCD sensor chip is analog and must be converted to either NTSC analog video for storage on VCR tape and viewing on a standard television monitor (resulting in a significant resolution loss), or converted to digital in order to write to a digital storage medium and display on a computer monitor. Presently, the major limitations of high resolution DFCs in digital mode are data transfer and storage rates. Each image consists of over 1 Mbyte of information, so data transfer to an inexpensive tape drive or disk is slow. This does not permit large image overlap as in NTSC video where a complete image is acquired every 1/30 s. Consequently, remote sensing applications of these sensors are currently limited to non-continuous data acquisition with some capability for image overlap.

One application which is well suited to monochrome stereo imaging is elevation determination using photogrammetric methods. Digital elevation models (DEMs) represent important information layers in geographic information systems. Advances are currently being made toward production of small scale DEMs using stereo satellite-based imaging such as SPOT (e.g., Chapman et al., 1991). The difficulties which have been encountered in these studies are primarily related to the line scanning technique employed by pushbroom scanners. Because each line of data has a different imaging geometry, the photogrammetric procedures for determining exposure station locations and relating them to ground survey data are more cumbersome than for the two-dimensional exposures provided by frame cameras. In addition, the large

> Photogrammetric Engineering & Remote Sensing, Vol. 60, No. 11, November 1994, pp. 1321–1326.

0099-1112/94/6011-1321\$3.00/0 © 1994 American Society for Photogrammetry and Remote Sensing

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ground pixel size of satellite images limits the precision of elevation determination.

The aim of this research is to use frame camera imagery in the development of large-scale digital elevation models. The project described in this paper was a feasibility study to assess whether elevations could be determined with sufficient accuracy to warrant continued research into large-scale elevation modeling using this imaging technique. The objectives were to (1) adapt a DFC to airborne imaging, (2) acquire overlapping digital imagery over a test site with suitable survey control and elevation variations, (3) perform relative and absolute orientation of the imagery using selected ground control points, and (4) determine the accuracy of elevations of other points within the imagery. The remainder of this paper provides a brief description of the methods and results obtained.

Methodology

Data Acquisition

Airborne image data were acquired on 29 October 1990. An area in the east end of Toronto, Canada was selected as a test site because it provided an elevation variation of approximately 50 m on former shorelines of Lake Ontario. The site consisted mostly of residential and small commercial land uses so there were abundant municipal horizontal and vertical survey control points. The frame camera imagery was acquired using the Kodak MEGAPLUS 1.4 camera. It is a black-and-white CCD sensor which incorporates an 8.98 mm by 7.04 mm chip with 1340 (H - horizontal) by 1037 (V vertical) photosites, each being 6.8 µm square. Twenty columns and one row at the top and bottom of the sensor are shielded to serve as a dark current reference so the actual number of active photosites is 1320 (H) by 1035 (V). A Nikon 15-mm focal length lens was used which provided a view angle of 33.3° (H) by 26.4° (V). A lens f-stop of between 8 and 11 was required for adequate exposure. The camera shutter was set at its maximum speed, 10 ms, to minimize image motion effects at low altitude, although it was recognized that some image motion would be inevitable with such a slow shutter. Images were acquired at three altitudes to evaluate the sensor's resolution capabilities in relation to varied ground coverage and pixel size. Two flight lines were flown from south to north at each altitude to provide evenly illuminated imagery with 15 percent sidelap. Table 1 lists the flight parameters for data acquisition. Analysis of the low altitude data for elevation determination is reported here.

Images acquired by the sensor in analog form were digitized by an 8-bit (256 grey level) A/D converter within the camera. Read-out was routed through an Imagraph HI*DEF 1280 by 1024 frame buffer (reducing the image size slightly but not the resolution) to a 100 Mbyte hard drive installed in a standard microcomputer. A hard drive was selected as the storage medium because the data writing speed was higher than for low cost tape media. Storage times were approximately 3.5 seconds per 1.3 Mbyte image. A program was written which permitted the capture and storage of images at user specified time intervals (P. Curran, personal communication, 1990). The appropriate intervals (Table 1) were calculated based on aircraft altitude and a velocity of 210 km/hr. given a requirement of 60 percent forward image overlap. The commands for image capture were entered in-flight using a standard microcomputer monitor and each captured image was viewed on a separate 1280 by 1024 monitor. The

TABLE 1.	FLIGHT PARAMETERS FOR	IMAGERY ACQUIRED	AT THREE	ALTITUDES.
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Altitude (m)	Ground Coverage (m) (H by V)	Pixel Size (m) (H by V)	Acquisi- tion Interval (s)	
1524	910 by 720	0.69 by 0.69	4.9	
3048	1820 by 1430	1.38 by 1.38	9.8	
4572	2730 by 2150	2.08 by 2.08	14.7	

camera, CPU, two monitors, and the camera power supply were mounted in a Piper Navaho aircraft (operated by the Ontario Ministry of Natural Resources, Provincial Remote Sensing Office). For the total set of six flight lines, 59 images were acquired (approximately 78 Mbytes) and stored on the hard disk.

Data Processing and Analysis

For the objective of evaluation of the sensor's potential in elevation determination, one pair of overlapping images from the 1524-m (5000-foot) flight line was selected. The data were viewed and manipulated using USGS MIPS image display and processing software. The two images were edge enhanced using a 3 by 3 Laplacian filter in order to improve pixel identification of suitable control and test points. Thirty image points dispersed as evenly as possible inside and outside the overlap region were visually selected for evaluation. They were typically located at intersections of sidewalks or roads and were distributed as uniformly as possible throughout the scene. Through a zoom function to enlarge the scale, the row and column pixel coordinates for each image point were determined. A field reconnaissance was conducted to identify the points as best as possible on the ground. Horizontal and vertical surveys were then carried out using standard leveling and traversing techniques to tie the ground points in with nearby Metropolitan Toronto horizontal and vertical control points.

To determine elevations in the overlapping DFC images. a bundle adjustment program (M. Chapman, personal communication, 1991) was used. It imposes the collinearity condition to perform a three-dimensional conformal coordinate transformation from the image space to the corresponding ground space. This is a common procedure in analytical photogrammetry, so it is not discussed in detail (e.g., see Moffitt and Mikhail (1980) or Ghosh (1987) for a complete derivation). Of the 30 image points, three were selected as control points and given a weight 1000 times the weight of the remaining points in the adjustment. This small number of control points was used in order to simulate an operational situation where only a few control points can be measured because of cost or difficulty in identification. Because ground survey data were available for all the points, approximately 20 combinations of control points which were easily identified in the images were tested in repeated program runs to determine the best solution. Lens focal length and principal point position were also required as inputs. At this time, a sensor calibration had not yet been performed (the sensor was on loan) so a focal length of 15.000 mm and a principal point location at the center of the sensor were assumed. The effects on the solution of varying these values by small amounts were evaluated to ensure that they were not grossly in error.

Evaluation of the accuracy of the bundle adjustment was straightforward; the survey data and adjusted position for each point were compared to determine residuals or error





(b)



Figure 1. Frame camera images of the Toronto Shoreline corresponding to the altitudes given in Table 1.

measurements. The optimum combination of control points was taken as that which produced the minimum root-meansquare (RMS) error for the elevations of all the points. In addition, repeated testing with various groups of control points allowed detection and removal of single points with consistently large residual values (outliers). Such points may have been incorrectly identified in the images and/or on the ground, or they may have been surveyed incorrectly.

Results

As an example of the image quality attainable with the ME-GAPLUS camera at three scales (see Table 1), Figures 1a to 1c show images of the Toronto shoreline taken from the low to high altitudes, respectively. The high altitude image is visibly affected by atmospheric haze which was quite strong. In addition, all the images are slightly degraded through reproduction using a laser printer and subsequent photographing. However, they illustrate the great potential of high resolution DFCs for detailed imaging while maintaining good ground coverage. For the purposes of elevation modeling, identification of ground points such as sidewalk intersections becomes difficult in the higher altitude imagery, but many other features are present which can easily be identified (road intersections, large building corners, etc.). With larger coverage, the probability of occurrence of such points is greater, so control and test point selection should not be more difficult at smaller scales in an urban area such as this (the precision of their measurement will of course be lower).

Figure 2 shows the overlapping images used for elevation determination. They are located just north of Figures 1a and 1b and they are included in the coverage of Figure 1c. Image motion of just less than one pixel (0.58 m at 210 km/ hr aircraft velocity) resulted in slight blurring of detail but its



Figure 2. Overlapping images of the test area (altitude = 1524 m).



effects were reduced through the edge enhancement procedure. The best bundle adjustment solution was obtained using point #s 20, 13, and 4 as control and eliminating six points which consistently had large residuals, indicating either ground survey or image location errors. The locations of the remaining points are shown in Figure 3. Table 2a gives the image point row-column coordinates (transformed to the right-hand coordinate system with center origin for use by the bundle adjustment algorithm) and their adjusted position while Table 2b gives the corresponding ground survey and adjusted positions. The projection used by the city of Toronto was a Modified Transverse Mercator (MTM) with 3° easting zones, Toronto being in zone 10, and a center meridian of one million feet. The x (easting) values in Table 2b have been converted to metres for consistency. The northings of the MTM projection as given in Table 2b are the same as UTM northings. The differences in the survey and adjusted data are listed as residuals in metres for the ground data and in 1/1000 of the pixel size for the image data. The RMS of the residuals is given in the bottom right of each table. For the ground coordinates, the control point residuals have been excluded from the RMS calculation because they were essentially fixed by their large relative weights.

The residuals show that the method has good potential for elevation determination and modeling. Elevation RMS was about 2.3 percent of the total variation in topography within the scene and is similar in dimension to the pixel size. The individual point residuals vary significantly, however, indicating a need for improvement in image point selection, ground point identification, and imaging conditions. The larger *y* residuals may have resulted from less precise image coordinate specification in the *y* direction due to the small amount of image motion which was present. Using other combinations of widely spaced control points, particularly ones which included corner points, resulted in higher residuals. Corner points were usually difficult to identify in the images or on the ground and were more likely to be affected by camera/lens distortions. Also, increasing the number of control points to five reduced the ground position residuals significantly but increased the image position residuals. This is because the ground model became more fixed so the image model had to be adjusted more in order to relate the two.

Discussion

Several points of discussion can be made regarding both the methodology and results achieved in this study. First, results were hindered by an absence of calibrated focal length, principal point location, and lens distortions. An inaccurate focal length deteriorates the collinearity of the rays for each image and produces inaccurate object space intersections of the rays from the two exposure stations (Moffitt and Mikhail, 1980). It was assumed to be 15.000 mm although this value is unlikely. An inaccurate principal point position causes relative translations between the image and object space (Moffitt and Mikhail, 1980). The center of the CCD chip is

TABLE 2a. RESULTS OF THE BUNDLE ADJUSTMENT SHOWING SELECTED IMAGE POINT COORDINATES, THEIR ADJUSTED COORDINATES, THE RESIDUAL ERRORS IN 1/000 pixel, and the RMS ERROR.

lmage 1	Image Coor- dinates (pixels)		Adjuste Coord (pix	d Image linates cels)	Difference (1/1000 pixel)	
Point	x	y	x	y	$V_{\rm x}$	V _y
1	517	-354	516.992	-354.008	-8.4	-8.0
4	-157	-45	-155.468	-44.636	1532.2	364.2
5	-351	-318	-350.981	-317.987	18.8	13.3
20	502	299	501.514	298.608	-486.5	-391.6
33	238	3	236.987	3.131	-1012.5	130.6
34	-32	79	-31.982	78.898	18.7	-101.7
44	-146	-245	-146.044	-246.020	-3.7	-20.1
45	-215	-480	-214.999	-479.992	1.5	7.8
46	139	-316	138.990	-315.993	-9.8	6.5
47	-84	-448	-83.995	-447.984	4.5	15.9
48	144	-373	144.007	-372.990	6.7	10.3
50	476	-187	475.996	-187.000	-4.3	0.4
51	370	-177	369.995	-176.999	-5.3	1.1
52	479	-301	478.995	-301.003	-5.2	-2.7
53	529	-310	528.993	-310.005 RMS	$-6.8 \\ 490.6$	-5.0 144.8

Image 2	Image Coor- dinates (pixels)		Adjuste Coord (pix	d Image linates cels)	Difference (1/1000 pixel)	
Point	x	y	x	У	$V_{\rm x}$	V_y
2	426	265	426.998	265.001	-1.6	3.5
3	-384	223	-383.989	222.989	11.4	-11.4
4	-295	-507	-295.297	-506.604	- 297.5	396.4
9	-59	396	-58.992	395.991	7.9	-9.5
11	264	268	263.993	268.000	-6.8	0.3
12	215	266	214.993	265.998	-6.6	-2.3
13	-12	264	-12.316	264.181	-316.2	181.4
14	-208	265	-207.997	264.990	2.7	-10.0
17	-216	97	-216.007	96.998	-6.7	-2.2
18	-65	69	-65.010	69.011	-10.3	10.8
20	371	-155	370.641	-155.531	-359.1	-531.3
33	103	-459	403.996	-459.123	995.6	-123.1
34	-170	-382	-170.021	-381.901	-21.1	98.7
				RMS	317.4	195.6

TABLE 2D. RESULTS OF THE BUNDLE ADJUSTMENT SHOWING THE POINT FIELD SURVEY POSITIONS, THEIR ADJUSTED COORDINATES, THE RESIDUAL ERRORS IN METRES, AND THE RMS ERROR. NOTE THAT THE X (EASTING) VALUES ARE FROM THE 3° MODIFIED TRANSVERSE MERCATOR PROJECTION USED BY THE CITY OF TORONTO.

	Groun	Ground Survey Coordinates (m)			Adjusted Ground Coordinates			Difference (m)		
Pt.	X	Ŷ	Z	X	Y	Ζ	V_{x}	Vy	V_z	
20	320380.660	4837580.730	107.848	320380.661	4837580.735	107.849	0.001	0.005	0.001	
13	320032.960	4837675.580	119.088	320032.961	4837675.580	119.088	0.001	0.000	0.000	
4	320143.820	4837167.040	94.718	320143.818	4837167.035	94.719	-0.002	-0.005	0.001	
1	320609.640	4837231.710	106.852	320610.084	4837233.488	106.996	0.444	1.788	0.144	
2	320266.970	4837823.070	129.425	320267.479	4837822.730	129.367	0.509	-0.339	-0.058	
3	319848.300	4837526.210	112.177	319845.864	4837526.739	112.795	-2.436	0.529	0.618	
5	320135.330	4836955.160	98.866	320134.057	4836951.813	99.589	-1.303	3.347	0.723	
9	319963.960	4837730.720	119.300	319962.138	4837731.293	119.704	-1.822	0.573	0.404	
11	320179.220	4837769.750	126.152	320180.150	4837770.277	126.264	0.930	0.527	0.112	
12	320153.940	4837751.980	124.852	320154.624	4837752.841	125.016	0.684	0.861	0.164	
14	319927.990	4837608.480	115.526	319926.809	4837609.553	115.858	-1.181	1.076	0.332	
17	319980.060	4837515.390	114.658	319980.764	4837516.252	114.587	0.704	0.862	-0.071	
18	320069.280	2837553.180	115.995	320071.553	4837552.615	115.772	2.273	-0.565	-0.223	
33	320339.850	4837329.100	102.456	320340.080	4837329.112	105.285	0.230	0.012	2.829	
34	320168.200	4837276.810	102.914	320168.584	4837277.013	105.805	0.384	0.203	2.891	
44	320220.370	4837058.880	93.005	320219.152	4837061.826	92.770	-1.218	2.946	-0.235	
45	320263.100	4836912.790	91.930	320263.576	4836911.634	92.159	0.476	-1.156	0.229	
46	320395.690	4837123.420	96.296	320397.543	4837123.383	96.449	1.853	-0.037	0.153	
47	320322.390	4836977.200	92.638	320323.164	4836974.719	93.055	0.744	-2.481	0.417	
48	320419.720	4837096.540	96.581	320419.712	4837094.616	96.724	-0.008	-1.927	0.143	
50	320532.960	4837308.890	100.242	320533.569	4837309.206	100.374	0.609	0.316	0.132	
51	320473.150	4837277.950	97.087	320473.941	4837278.242	97.210	0.791	0.292	0.123	
52	320571.970	4837247.970	106.097	320572.433	4837248.766	106.219	0.463	0.796	0.122	
53	320601.010	4837259.760	107.922	320601.494	4837260.994	108.084 RMS	0.484 1.138	1.234 1.390	0.162 0.927	

specified by the manufacturer to an accuracy of only ± 100 pixels relative to the optical axis; it was assumed to lie at the center pixel relative to the overall sensor dimensions (active and inactive photosites) because this position produced the best results of several positions tested. For the wide angle lens used, distortion may cause positional errors in image point selection, particularly near the scene boundaries, therefore decreasing the precision of the model fit. Currently, research being initiated to evaluate the sensor in elevation modeling (King and Chichagov, 1993) is incorporating camera calibration to improve the precision of the technique.

Second, in data acquisition, there were four difficulties, all of which can be ameliorated. (1) Efforts were made to obtain 60 percent overlap between image frames but, due to high tailwinds, the air speed was greater than ground speed and only about 52 percent overlap was achieved. This reduced the area of the stereo model coverage and the availability of usable control points. (2) The date of imaging was constrained by weather and aircraft availability. In late October, most deciduous leaves were still on the trees in Toronto and the sun was quite low, producing an abundance of shadows in the imagery. This severely hindered the process of image point identification and limited the number of available points for selection. Flying must be conducted without the presence of deciduous foliage but with as high a sun elevation as possible (e.g., mid-spring) or under high overcast conditions. (3) The process of visually identifying points in the pair of images was tedious and relied on the analysts' subjective judgement through interpolation along grey-level gradients which defined object edges. Image motion was an additional effect which reduced edge sharpness. Consequently, each image point's location could only be specified with a precision of approximately ± 1 pixel in the x and perhaps ± 1 to 2 pixels in the *y* (flight) direction using these methods. Selection of control points, the most critical step in the procedure, must have greater precision than this. Future research will incorporate a faster camera shutter and methods for semi-automated or automated image point selection using correlation techniques. (4) Lastly, once the points had been specified in the images, the field crew had to identify them on the ground before surveying their horizontal and vertical coordinates. This was very difficult as the digital pixels at this scale rarely reproduce objects exactly; although the ground survey was conducted with high precision, the placement accuracy of some of the selected ground points may have been low. In practice, only three points need to be surveyed accurately in order to fit the model. The additional points receive minor weights in the model (1/1000 here) and are required only if a complete accuracy assessment is desired.

It is evident that digital frame camera imaging for elevation determination has great potential. However, the results reported here apply the specific test conditions of an urban scene with only a moderate elevation variation imaged at low altitude. Research is needed to determine the universal applicability of the method. For example, future work will be conducted to (1) assess the effects of varying pixel size on elevation determination using data from all three flight altitudes, (2) determine the capability of the method in areas where available ground control is less precise or less numerous (i.e., outside urban areas), and (3) extend the method from point elevation determination to large-scale elevation modeling using blocks of imagery.

Conclusions

The emerging technology of digital frame cameras has advantages of higher resolution and radiometric quality than NTSC-

based video systems and of better geometric quality than line scanners. It is well suited to both photogrammetric applications and multispectral imaging. Digital frame camera technology combined with analytical photogrammetric techniques has potential in the determination of spot terrain elevations and as a data source for digital elevation models. The results reported here accompanied by the outlined improvements suggest that determination of elevation using DFC imaging and analytical photogrammetric techniques should attain accuracies suitable for many municipal and rural mapping applications. With continued research, such methodology can be used to produce large-scale digital elevation models for integration with other data in a GIS framework.

Acknowledgments

The authors gratefully acknowledge the support of the National Research Council of Canada, Cadix Inc. of Toronto (Canadian distributors of the Kodak MEGAPLUS camera), and the Ontario Ministry of Natural Resources, Provincial Remote Sensing Office. We would also like to thank Dr. M. Chapman, Department of Geomatics Engineering, University of Calgary, for advice on the use of his bundle block adjustment program, and Gladwin Henry of Ryerson Polytechnical Institute in Toronto for additional contributions to this project.

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(Received 17 March 1992; revised and accepted 2 February 1993; revised 24 March 1993)

Introduction to the PHYSICS AND TECHNIQUES OF REMOTE SENSING

by: Charles Elachi

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